Drainage Report

5000 West Mercer Way – Moran Residence

Mercer Island, WA

Prepared for

Edward & Catherine Moran 5000 West Mercer Way Mercer Island, WA 98040

Prepared by

JMJ TEAM PO Box 2066 Sumner, WA 98390 206.596.2020 Justin Jones, PE

May 19, 2023



PROJECT ENGINEER'S CERTIFICATION

"I hereby state that this Drainage Control Plan for the Moran Residence has been prepared by me or under my supervision and meets minimum standard of care and expertise which is usual and customary in this community for professional engineers. I understand that Pierce County does not and will not assume liability for the sufficiency, suitability, or performance of drainage facilities prepared by me."

Justin Jones, PE





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PROJECT OVERVIEW AND MAPS

The Moran Residence is located along West Mercer Way on Mercer Island. The project includes the construction of a new single-family residential building, driveway, and site retaining walls. The projects stormwater approach is to implement detention as the Best Management Practice (BMP).



EXISTING CONDITIONS SUMMARY

The Moran Residence is an undeveloped 0.42 Acre site with grass and tree vegetation covering most the property. The site has steep slopes that slope from east to west.

The existing project site is pervious. The total impervious coverage allowed for this project is 35% or 6,403 SF.

PROPOSED CONDITIONS SUMMARY

The Moran Residence project proposes a house, permeable paver walkaway, concrete driveway, and site retaining walls. Site improvements include the construction of the improvements, clearing and grading, and utility service connections for storm detention, sewer, water, power and communication.

Stormwater management was evaluated for both the building roof areas, and the concrete driveway. Detention has been selected to manage stormwater runoff from the site. Roof leaders will route stormwater along the building and connect to a Type 2 catch basin. Runoff from the driveway will be collected through the Type 2 catch basin located north of the house. Stormwater will be collected in the Type 2 catch basin prior to entering the detention tanks. A control structure will be installed to ensure stormwater flows do not exceed 0.15 CFS, flows from the control structure will be routed to an existing Type 1 catch basin located at the corner of W Mercer Way and the private gravel road to the north of the site. Flows from the driveway will be collected using a trench drain located at the bottom of driveway and will be routed to a Grinder Pump System which will be pump the driveway runoff back to the proposed detention tanks. Footing drains will be installed along the footings of the wall and proposed house, the drains will be routed to the 6" PVC pipe located in the proposed driveway and be conveyed to the existing type 1 catch basin. Stormwater collected from the shoring wall and foundation footings will not be routed through the detention tanks.

The impervious areas will be 25 percent of the entire site. Below is a summary of the proposed lot coverage.

Pro	posed Lot Coverage	
		Pervious Areas
	Impervious Areas (SF)	(SF)
Proposed House	2,664	
Proposed Driveway	1,312	
Proposed Retaining Walls	63	
Permeable Pavers		116
Landscaping/Vegetaion		13,722
Totals	4,039	13,838
Lot Size	18,295	
Max Allowed Impervious		
Coverage	35% (6,403 SF)	
Impervious Lot Coverage	25%	

LOT COVERAGE

SUMMARY OF MINIMUM REQUIREMENTS

The 2014 Stormwater Management Manual for Western Washington describes the minimum requirements for a new development project. Using the flowchart below, Minimum Requirements 1-5 apply to the project site.



Volume I – Minimum Technical Requirements – December 2014 2-5

MINIMUM REQUIREMENT 1: PREPARATION OF STORMWATER SITE PLANS

Stormwater Site Plan drawings are submitted with this Permit.

MINIMUM REQUIREMENT 2: CONSTRUCTION STORMWATER POLLUTION PREVENTION

A Temporary Erosion and Sediment Control Plan is included with this Civil Permit. Construction Stormwater Pollution Prevention measures may include: storm drain inlet protection; construction entrance; silt fence and vegetative filtration. See "Temporary Erosion & Sediment Control Plan" in Appendix A for details.

MINIMUM REQUIREMENT 3: SOURCE CONTROL OF POLLUTION

Source control BMPs will be implemented to minimize stormwater contamination and help comply with the 2014 Stormwater Management Manual for Western Washington Manual. BMP's for the project may include:

• Inspect and clean treatment BMPs, conveyance systems, and catch basins as needed, and determine necessary O & M Improvements.

MINIMUM REQUIREMENT 4: PRESERVATION OF NATURAL DRAINAGE SYSTEMS AND OUTFALLS

Natural drainage for the site is overland flow from east to west flowing into and existing ditch located along West Mercer Way. Stormwater will be conveyed to detention tanks located in northern portion of the site, stormwater will then outfall to an existing ditch located to the west along West Mercer Way.

MINIMUM REQUIREMENT 5: ONSITE STORMWATER MANAGEMENT

The Moran project site is 18,295 SF and will be 25% impervious after construction. Several stormwater management techniques were studied for the roof and driveway areas.

- <u>Roofs:</u>
 - Bioretention/Rain Gardens were deemed infeasible based on the geo-tech report, due to steep slopes of the site and impermeable soils at shallow depths infiltration was deemed infeasible.
 - Downspout Dispersion Systems were evaluated and deemed infeasible due to the steepness of the site and site constraints to achieve minimum flow paths.
 - Perforated stub connections were considered infeasible based on the geo-tech report, due to steep slopes of the site and shallow impermeable soils making infiltration infeasible.
 - 65/10 dispersion was deemed to be infeasible as the existing property does not maintain 65% of the site area in a native condition.
 - A Dispersion Trench was considered infeasible due to site constraints and not having adequate space for the placement of a dispersion trench.
 - Infiltration trenches were evaluated and were determined infeasible due to the impermeable soils located on site, based on findings found in the geo-tech report.
 - Detention was evaluated and deemed feasible as the BMP for project site, roof runoff will be collected and routed to on site detention system.

- Other Hard surfaces:
 - Bioretention/Rain Gardens were deemed infeasible based on the geo-tech report, due to steep slopes of the site and impermeable soils at shallow depths infiltration was deemed infeasible.
 - 65/10 dispersion was deemed to be infeasible as the existing property does not maintain 65% of the site area in a native condition.
 - Infiltration trenches were evaluated and were determined infeasible due to the impermeable soils located on site, based on findings found in the geo-tech report.
 - Permeable Pavement was deemed infeasible due to impermeable soils located on site. Making infiltration infeasible.
 - Sheet flow dispersion was deemed infeasible due to site constraints, the site slope is greater than 15%.
 - Concentrated flow dispersion was evaluated and deemed infeasible due to the steep site slopes and site constraints that minimum flow paths can't be met.
 - Detention was reviewed and deemed feasible to manage runoff from the proposed driveway. Runoff will be collected through a Type 2 catch basin and routed to the onsite detention tank systems.

LID standards were evaluated, and the Moran residence does not meet the minimum LID thresholds. The projects proposes more than 2,000 SF of impervious area and has more than a net 500 SF impervious area increase to the project site. Therefore, the project is required to use onsite detention. Detention was sized using the City of Mercer Island Detention Sizing Handout (See Appendix B). Using the control structure, flows leaving the site will not exceed 0.15 CFS of the predeveloped flows of the site. Site flows will be routed to a Type 1 catch basin located on the west corner of the site and outfall to an existing ditch located along West Mercer Way. A Grinder pump system will be used to convey the part of the driveway runoff to the detention tank, see Appendix C for pump sizing.

APPENDIX A



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EASEMENTS

TESC NOTES

- Contractor to install temporary erosion and sediment control measures as necessary to ensure stormwater leaving the site is free of settleable solids.
- Roads shall be cleaned thoroughly as needed to protect stormwater infrastructure and downstream water resources. Sediment shall be removed from roads by shoveling or pickup sweeping and be transported to a controlled sediment disposal area.
- Install strom drain inlet protection in all existing catch basins within the project vicinity per City of Mercer Island Detail 4.2.8.
- Install Stabilized Construction Entrance per City of Mercer Island Detail 4.1.1.
- Install Silt Fence as necessary. See City of Mercer Island Detail 4.2.12.
- Install straw bale barriers, wattles and other TESC measures as necessary.
- Exposed soils shall be watered as necessary to prevent dust from leaving the site.
- Contractor to mark clearing limits with lath and flagging.
- Concrete handling and equipment washing shall in accordance with DOE BMP C151.

GENERAL NOTES

See Tree Inventory Tables in Arborist Report included in this submittal.

CONSTRUCTION NOTES

- The lawn and landscape areas are required to provide Post-Construction Soil Quality and Depth in accordance with BMP T5.13. The project civil engineer must provide a letter of certification to ensure that lawn and landscape areas are Depth Requirements specified on the approved plan set prior to final inspection of the project.
- POST-CONSTRUCTION SOIL MANAGEMENT
- 1. Retain & Protect Native Vegetation and Soil 1.1. Identify Areas of the site that will not be disturbed construction. Fence areas to prevent impacts during construction.
- 2. loosen Compacted Subsoil
- 2.1. In Areas Compacted by Construction Traffic Scarify the top 4-inches of subsoil. Use a Cat-mouted Ripper, tractor-mounted disc, or tiller to mix the first lift of topsoil into the subsoil. USe the equipment listed to scarify soils to a depth of 12-inches before tilling in at least 8-inches of compost.
- 3. Restore Soils that are Disturbed During Construction 3.1. Stockpile and reuse existing topsoil (amend if needed to meet 5% organic matter content for turf areas; 10% organic matter content for planting beds).
- 4. Add Mulch to Planting Beds
- 4.1. Spread mulch (coarse bark or wood chips) in the spring or fall (after planting) to control weeds, reduce the need for irrigation and prevent erosion). Apply 1 to 2 inches of mulch on planting beds and around shallow-rooted annuals. Apply 2 to 4 inches of mulch around trees and woody perennials, but make sure to keep mulch 2-3 inches away from tree trunks.
- 5. Protect Restored Soils from Erosion anad Re-Compaction 5.1. Prevent runoff from roads or open slopes onto amended soil areas. Compost blankets are an approved erosion control Best Management Practice (BMP) that can be used during construction and then tilled into existing soil at the end of the construction process prior to planting. Once soils have been amended, vehicle traffic should be prohibited to prevent recompilation from occurring.

LEGEND

Trees to be Removed



Protect and Maintain Existing Trees

CALL TWO BUSINESS DAYS

-800-424-5555 UTILITIES UNDERGROUND LOCATION CENTER

BEFORE YOU DIG 1-800-424-5555

Edward & Catherine Moran 5000 West Mercer Way Mercer Island, WA 98040
Architect:
Plan One Fine Home Design 5125 47th Ave S Seattle, WA 98118 206-612-8511
Engineer: JITEAM Justin Jones, PE PO Box 2066

Owner/Developer:

Project: Moran Residence

Sumner, WA 98390

(206) 596-2020

ONE INCH AT FULL SCALE. IF NOT, SCALE ACCORDINGLY





Existing Site & TESC Plan

PROJ.NO: 1	576001
DATE: Ma	ay 19, 2023
DRAWN BY:	DESIGN BY:
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LEGEND



Proposed Concrete

Proposed Concrete with Raked Surface

Owner/Developer:

Proposed Permeable Pavers

Landscaping/Native Vegetation

GENERAL NOTES

- See Detail on Sheet C-05 for Standard Concrete Section.
- See Detial on Sheet C-05 for Permeable Paver Section.
- Driveway Slopes over 20.0% add a Raked Surface Finish to increase Traction.

LOT COVERAGE

Pro	posed Lot Coverage	
	Impervious Areas (SF)	Pervious Areas (SF)
Proposed House	2,664	
Proposed Driveway	1,312	
Proposed Retaining Walls	63	
Permeable Pavers		116
Landscaping/Vegetaion		13,722
Totals	4,039	13,838
Lot Size	18,295	
Max Allowed Impervious		
Coverage	35% (6,403 SF)	
Impervious Lot Coverage	22%	

Lot Coverage Calculations

A. Gross Lot Area	18,295	Square Feet
B. Net Lot Area	16,865	Square Feet
C. Allowed Lot Coverage Area	5,060	Square Feet
D. Allowed Lot Coverage	35	% of Lot
E. Existing Lot Coverage:		
1. Main Structure Roof Area	0	Square Feet
2. Accessory Building Roof Area	0	Square Feet
3. Vehicular Use (driveway, paved access easements		
[portion used by the lot for access], parking)	0	Square Feet
4. Covered Patios and Covered Decks	0	Square Feet
5. Total Existing Lot Coverage Area (E1 + E2 + E3 + E4)	0	Square Feet
F. (Total Lot Coverage Area Removed)	0	Square Feet
G. Proposed Adjustment for Single Story (Area)	0	Square Feet
H. Proposed Adjustment for Flag Lot	0	Square Feet
I. Total New Lot Coverage Area:		
1. Main Structure Roof Area	2,239	Square Feet
2. Accessory Structure Roof Area	0	Square Feet
3. Vehicular Use (driveway, paved access easements		
[portion used by the lot for access], parking)	1,912	Square Feet
4. Covered Patios and Covered Decks	425	Square Feet
5. Total New Lot Coverage Area (I1 + I2 + I3 + I4)	4,576	Square Feet
J. Total Project Lot Coverage Area = (E5 - F) + I5	4,576	Square Feet
K Proposed Lot Coverage Area = $(1/B) \times 100$	27.1	% of Lot

Hardscape Calculations 18,295 Square Feet 16,865 Square Feet 0 Square Feet A. Gross Lot Area B. Net Lot Area C. Area Borrowed from Lot Coverage D. Allowed Hardscape Area = 9% of lot area + C 0 % of Lot E. Allowed Hardscape Area 0 Square Feet F. Total Existing Hardscape Area: 1. Uncovered Decks 0 Square Feet 2. Uncovered Patios 0 Square Feet 3. Walkways 0 Square Feet 4. Stairs 0 Square Feet 0 Square Feet 5. Rockeries and Retaining Walls 6. Other 0 Square Feet 7. Total Existing Hardscape Area (F1 +F2 + F3 + F4 + F5 + F6)) Square Feet G. (Total Hardscape Area Removed) 0 Square Feet H. Total New Hardscape Area: 0 Square Feet 0 Square Feet 1. Uncovered Decks 2. Uncovered Patios 119 Square Feet 44 Square Feet 3. Walkways 4. Stairs 5. Rockeries and Retaining Walls 70 Square Feet 6. Other 0 Square Feet 7. Total New Hardscape Area (H1 + H2 + H3 233 Square Feet 233 Square Feet + H4 + H5 + H6) I. Total Project Hardscape Area = (F7 - G) + H7 J. Total Project Hardscape Area = (I/B) x 100 1.4 Square Feet

Lot Slope Calcula	ations			Square Feet
Highest Elevation Point of Lot:	222	Feet	Lot Area	18,295
Lowest elevation Point of Lot:	184	Feet	Max Allowable Gross Area =	
Elevation Difference:	38	Feet	(Lot Area)*(40%)* (85%)	6,220
Horizontal distance Between			Max Allow Area per Floor =	
High and Low Points:	127	Feet	(Max Allowable Gross	
Lot Slope	29.9%	%	Area)/2	3,110

	Gross	Floor Area Calcula	tions	
	Existing Area	Removed Area	New/Addition Area	Total
Building Area	(Square Footage)	(Square Footage)	(Square Footage)	(Square Footage
Upper Floor	0	0	1848.8	1848.
Main Floor	0	0	1402.7	1402.
Gross Basement Area	0	0	0	
Garage/Carport	0	0	635.5	635.
Total Floor Area	0	0	3887	388

Edward & Catherine Moran 5000 West Mercer Way Mercer Island, WA 98040 Architect: Plan One Fine Home Design 5125 47th Ave S Seattle, WA 98118 206-612-8511 Engineer: JUTEAM Justin Jones, PE PO Box 2066 Sumner, WA 98390 (206) 596-2020 Project: Moran Residence ONE INCH AT FULL SCALE. IF NOT, SCALE ACCORDINGLY ALLA A 41829 STONAL F 05-19-2023 REV DATE DESCRIPTION SHEET TITLE. Site & Grading Plan 1576001 PROJ. NO: May 19, 2023 DRAWN BY: DESIGN BY: SHEET NUMBER. C-02



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CONSTRUCTION NOTES

Sanitary Sewer Line

Water Line

Power Conduit

Stormwater Line

- ASTM 3034 SDR 35 PVC pipe, fused solid wall HDPE, schedule 40 ABS, DIP or CIP (up to 8 ft. depth). Over 8 ft. depth and slopes more than 20%, DIP, CIP, or fused solid wall HDPE are required.
- Bedding material for open cut construction must be pea gravel, sand, control density fill (CDF), or 5/8" minus C.R.
- Select backfill material shall be 5/8" minus C.R. or control density fill (CDF).
- Imported backfill material shall be bank run gravel or pit run gravel from an approved supplier meeting APWA/WSDOT gradation specifications. Not allowed in right-of-way.
- Rubber gaskets must be used when appropriate.
- Rigid couplings must be used forconnections to existing stubs in right-of-way.
- A stainless steel strap and saddle (Romac) must be used for coring.
- 1" Water Meter Installation see City of Mercer Island Detail on sheet C-06.
- Tapping Tee Installation see City of Mercer Island Detail on sheet C-06.
- The lawn and landscape areas are required to provide Post-Construction Soil Quality and Depth in accordance with BMP T5.13. The project civil engineer must provide a letter of certification to ensure that the lawn and landscape areas are meeting the Post-Construction Soil Quality and Depth Requirements specified on the approved plan set prior to final inspection of the project.

GENERAL NOTES

- Water Service laterals shall have a minimum cover of 12 inches.
- Roof leader locations to be verified by contractor prior to construction.
- Storm pipes to maintain a minimum cover of 1.5' from finish grade.
- Storm pipes to be SDR 35 PVC piping.
- Sanitary Sewer laterals to be soild wall HDPE piping.
- Saniatry Sewer Laterals to mantain a minimum cover of 3.0' from finish grade.
- Pipes entering and exiting catch basins a tee section or bent elbow must be installed for spill control.
- Power conduit shall maintain a minimum cover of 2.0' from finish grade.
- Franchise utilities shown on this plan are not reviewed or approved by the City of Mercer Island.
- The TV inspection of the existing side sewer to the City sewer main on W Mercer Way is required prior to any work related to the side sewer. If the result of the TV inspection is not in satisfactory condition, as determined by the City of Mercer Island Inspector, the replacement of the existing side sewer is required.
- An emergency on-site, back-up power supply and an external alarm system for system failure and high water level indicator are required for the pump system.
- Private property owners shall be responsible for any and call claims for injuries and damage due to the operation or non-operation of the pump system.
- Stormwater Cleanout lid per WSDOT Standard Plan B-85.40-00

Owner/De	veloper:		
Edward	& Catherine	Moran	
5000 We Mercer I	est Mercer V sland, WA S	Way 98040	
Wereeri			
Architect:			
Plan One	e Fine Home	e Design	
5125 471 Seattle,	th Ave S WA 98118		
206-612	-8511		
Engineer:			
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Tree Protection Fencing and Limits of Clearing, Grading, and Excavation Steep Slope Buffer

Proposed Water Line

Proposed Storm Line

Proposed Drain Line

Proposed Storm Force Main

Proposed Sanitary Sewer Line

Proposed Power Line

Large Tree to be Removed

Exceptional Tree to be Removed

Non-Significant Tree to be Removed

CONSTRUCTION NOTES

All grading and excavation work within the limits of disturbance as indicated by cross-hatching shall be monitored by project arborist.

2 Pneumatic air or hydro excavation to be utilized at foundation locations in conflict with critical root zones and under the supervision of project arborist.

3 Utility trenching to be done with hydro or pneumatic methods or by hand excavation if recommended by and under supervision of project arborist

TREE PROTECTION FENCING AND

- 1. 6' H Chain Link, wire mesh, or similar open rigid material (No
- 2. Must be installed prior to demolition or ground disturbance
- 3. Kept in place for the duration of construction
- 4. No soil disturbance or activity allowed within fenced area; ma
- 5. Modifications of these requirements by approval of SDCI Plar
- 6. If roots greater than 2 inch found outside of fencing, protect excavation and, if necessary, cut cleanly and keep moist
- 7. Use 3 inches or deeper wood chip mulch within tree protecti as well for all trees impacted within their limits of disturbanc

VEGETATION PROTECTION

- 1. Orange mesh or similar open material
- 2. Minimize construction zone
- 3. Protect vegetation outside construction zone with fencing as s
- 4. Use 3 inches or deeper wood chip mulch outside fenced areas protect feeder roots

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CALL TWO BUSINESS

Owner/Developer:

Edward & Catherine Moran 5000 West Mercer Way Mercer Island, WA 98040

Architect:

Engineer:

Project:

Plan One Fine Home Design 5125 47th Ave S Seattle, WA 98118 206-612-8511

J Justin Jones, PE PO Box 2066

Sumner, WA 98390 (206) 596-2020

Moran Residence

ONE INCH AT FULL SCALE. IF NOT, SCALE ACCORDINGLY









Figure 4.1.1 – Stabilized Construction Entrance





Figure 4.2.12 - Silt Fence

	Owner/Developer:
	Edward & Catherine Moran 5000 West Mercer Way
	Mercer Island, WA 98040
	Architect:
	Plan One Fine Home Design
	Seattle, WA 98118 206-612-8511
	Engineer:
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	Justin Jones, PE
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	Project: Moran Residence
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TREE PROTECTION AREA (TPZ)

KEEP OUT!

DO NOT REMOVE OR ADJUST THE APPROVED LOCATION OF THIS TREE PROTECTION AREA

Trees enclosed by this fence are protected and are subject to the conditions of the tree permit. Violation of tree conditions may lead to:

Notes

- 1. Correction Notices or Stop Work Orders until compliance is achieved
- 2. RE Inspection Fees

KEEP OUT TREE PROTECTION

AREA

3. Arborist reports recommending mitigation

Crown drip line or other limit of Tree Protection area. See

Site/Utility Plan for fence alignment.

- 1. No pruning shall be preformed unless under the direction of an arborist
- 2. No equipment shall be stored or operated inside the protective fencing including during fence installation and removal
- 3. No storage of materials shall occur inside the protective fencing
- 4. Refer to Site/Utility Plan for allowable modifications to the tree protection area.
- 5. Unauthorized activities in tree protection area may require evaluation by private arborist to identify impacts and mitigation required
- Exposed roots: For roots > 1" damaged during construction, make a clean straight cut to remove damaged portion and inform City Arborist



2" x 6" steel posts or approved equal

Maintain existing grade with the tree protection fence unless otherwise indication on the plans







	Owner/Developer:
	Edward & Catherine Moran
	5000 West Mercer Way Mercer Island, WA 98040
	Architect:
	Plan One Fine Home Design 5125 47th Ave S
	Seattle, WA 98118 206-612-8511
	Engineer:
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	Project: Moran Residence
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DRAWN BY: ADAM COCHRAN

FOR STORMWATER USE

Washington State Department of Transportation



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	Owner/Developer:
	Edward & Catherine Moran 5000 West Mercer Way Mercer Island, WA 98040
	Architect:
	Plan One Fine Home Design 5125 47th Ave S Seattle, WA 98118 206-612-8511
	Engineer: JTEAM Justin Jones, PE
	PO Box 2066 Sumner, WA 98390 (206) 596-2020
	Project: Moran Residence
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APPENDIX B

Table 1

ON-SITE DETENTION DESIGN FOR PROJECTS BETWEEN 500 SF AND 9,500 SF NEW PLUS REPLACED IMPERVIOUS SURFACE AREA

				-						
		Detenti	on Pipe	Lowest Orifice		Distance from	Outlet Invert	Second Orifice		
New and Replaced		Lengt	:h (ft)	Diamet	er (in) ⁽³⁾	to Second	to Second Orifice (ft)		ter (in)	
Impervious Surface Area	Detention Pipe	B soils	Capila	P soils	Casila	P soils	Capila	B soils	Casila	
(sf)	Diameter (in)	D SOIIS	C SOIIS	D SOIIS	C SOIIS	D SOIIS	C SOIIS	D SOIIS	C SOIIS	
	36"	30	22	0.5	0.5	2.2	2.0	0.5	0.8	
500 to 1,000 sf	48"	18	11	0.5	0.5	3.3	3.2	0.9	0.8	
	60"	11	7	0.5	0.5	4.2	3.4	0.5	0.6	
	36"	66	43	0.5	0.5	2.2	2.3	0.9	1.4	
1,001 to 2,000 sf	48"	34	23	0.5	0.5	3.2	3.3	0.9	1.2	
	60"	22	14	0.5	0.5	4.3	3.6	0.9	0.9	
	36"	90	66	0.5	0.5	2.2	2.4	0.9	1.9	
2,001 to 3,000 sf	48"	48	36	0.5	0.5	3.1	2.8	0.9	1.5	
	60"	30	20	0.5	0.5	4.2	3.7	0.9	1.1	
	36"	120	78	0.5	0.5	2.4	2.2	1.4	1.6	
3,001 to 4,000 sf	48"	62	42	0.5	0.5	2.8	2.9	0.8	1.3	
	60"	42	26	0.5	0.5	3.8	3.9	0.9	1.3	
	36"	134	91	0.5	0.5	2.8	2.2	1.7	1.5	
4,001 to 5,000 sf	48"	73	49	0.5	0.5	3.6	2.9	1.6	1.5	
	60"	46	31	0.5	0.5	4.6	3.5	1.6	1.3	
	36"	162	109	0.5	0.5	2.7	2.2	1.8	1.6	
5,001 to 6,000 sf	48"	90	59	0.5	0.5	3.5	2.9	1.7	1.5	
	60"	54	37	0.5	0.5	4.6	3.6	1.6	1.4	
	36"	192	128	0.5	0.5	2.7	2.2	1.9	1.8	
6,001 to 7,000 sf	48"	102	68	0.5	0.5	3.7	2.9	1.9	1.6	
	60"	64	43	0.5	0.5	4.6	3.6	1.8	1.5	
	36"	216	146	0.5	0.5	2.8	2.2	2.0	1.9	
7,001 to 8,000 sf	48"	119	79	0.5	0.5	3.8	2.9	2.2	1.7	
	60"	73	49	0.5	0.5	4.5	3.6	2.0	1.6	
	36"	228	155	0.5	0.5	2.8	2.2	2.1	1.9	
8,001 to 8,500 sf ⁽¹⁾	48"	124	84	0.5	0.5	3.7	2.9	1.9	1.8	
	60"	77	53	0.5	0.5	4.6	3.6	2.0	1.6	
	36"	NA ⁽¹⁾	164	0.5	0.5	NA ⁽¹⁾	2.2	NA ⁽¹⁾	1.9	
8,501 to 9,000 sf	48"	NA ⁽¹⁾	89	0.5	0.5	NA ⁽¹⁾	2.9	NA ⁽¹⁾	1.9	
	60"	NA ⁽¹⁾	55	0.5	0.5	NA ⁽¹⁾	3.6	NA ⁽¹⁾	1.7	
	36"	NA (1)	174	0.5	0.5	NA ⁽¹⁾	2.2	NA (1)	2.1	
9,001 to 9,500 sf ⁽²⁾	48"	NA ⁽¹⁾	94	0.5	0.5	NA ⁽¹⁾	2.9	NA ⁽¹⁾	2.0	
	60"	ΝΔ ⁽¹⁾	58	0.5	0.5	ΝΔ ⁽¹⁾	3.7	ΝΔ ⁽¹⁾	1.7	

Notes:

• Minimum Requirement #7 (Flow Control) is required when the 100-year flow frequency causes a 0.15 cubic feet per second increase (when modeled in WWHM with a 15-minute timestep). Breakpoints shown in this table are based on a flat slope (0-5%). The 100-year flow frequency will need to be evaluated on a site-specific basis for projects on moderate (5-15%) or steep (> 15%) slopes.

- Soil type to be determined by geotechnical analysis or soil map.
- Sizing includes a Volume Correction Factor of 120%.
- Upper bound contributing area used for sizing.
- ⁽¹⁾ On Type B soils, new plus replaced impervious surface areas exceeding 8,500 sf trigger Minimum Requirement #7 (Flow Control)
- ⁽²⁾ On Type C soils, new plus replaced impervious surface areas exceeding 9,500 sf trigger Minimum Requirement #7 (Flow Control)
- ⁽³⁾ Minimum orifice diameter = 0.5 inches
- in = inch
- ft = feet
- sf = square feet

Basis of Sizing Assumptions:

Sized per MR#5 in the Stormwater Management Manual for Puget Sound Basin (1992 Ecology Manual) SBUH, Type 1A, 24-hour hydrograph 2-year, 24-hour storm = 2 in; 10-year, 24-hour storm = 3 in; 100-year, 24-hour storm = 4 in Predeveloped = second growth forest (CN = 72 for Type B soils, CN = 81 for Type C soils) Developed = impervious (CN = 98) 0.5 foot of sediment storage in detention pipe Overland slope = 5%



t Merce	r Way	PREPARED BY:	Justin Jones	
and, W	4	PHONE:	206-596-2020	
		DATE:	05/05/2023	
' ⊃Ε	DETENTION PIPE LENGTH (F	т): <u>73</u>	Orifice #1 dia $\frac{0.5}{1.6}$ inch, elev Orifice #2 dia $\frac{1.6}{1.6}$ inch, elev	185.19' 190.93'

CES (206–275–7605) 24 HOURS IN ADVANCE FOR A DETENTION RE BACKFILLING AND FOR FINAL INSPECTIONS.
ATION AND MAINTANANCE OF DRAINAGE SYSTEMS ON PRIVATE JTY OF THE PROPERTY OWNER. MATERIAL ACCUMULATED IN THE REMOVED FROM CATCH BASINS TO ALLOW PROPER OPERATION. FICE MUST BE KEPT OPEN AT ALL TIMES.
D PROTECTIVE TREATMENT SHALL BE IN ACCORDANCE WITH SECTION ISDOT STANDARD SPECIFICATION FOR ROAD, BRIDGE, AND MUNICIPAL IRSION. SUCH MATERIALS INCLUDE THE FOLLOWING, LINED VE PIPE (LCPE), ALUMINIZED TYPE 2 CORRUGATED STEEL PIPE AND D DESIGNATIONS M274 AND M36), CORRUGATED OR SPIRAL RIB FORCED CONCRETE PIPE. CORRUGATED STEEL PIPE IS NOT ALLOWED.
OT BE CONNECTED TO THE DETENTION SYSTEM.

APPENDIX C

Technical Memorandum

Project:	5000 West Mercer Way, WA 98040 Mordan Residence	From:	Justin Jones, PE
RE:	Storm Drainage Report - Driveway Pump System	Date:	March 21, 2023



03/21/2023

Introduction

This memo presents the criteria and methodology used for sizing the storm pump and force main conveying the driveway runoff to the detention pond. The proposed project site is a single-family project with a proposed 2,664 SF house, 1,312 SF driveway, 63 SF of retaining walls, and 119 SF of permeable paver path. The project adds more than 2,000 SF, but less than 5,000 SF of new plus replaced hard surfaces, therefore the runoff from the pollution generating surfaces does not have to be treated. An underground detention tank with a flow control structure will be used to manage stormwater runoff flows. The detention tank and control structure were sized per the City of Mercer Island on-site detention design requirements.

Runoff from part of the driveway will be conveyed to the detention tank using gravity flow through catch basins and storm lines. The remaining driveway runoff will be collected using a trench drain and pumped to the detention tank using a duplex pump system in a fiberglass basin. The pump system was sized based on the developed 100-year peak storm discharge rate, 0.009 cfs, for the portion of the driveway runoff that needs to be pumped to the detention tank, see driveway drainage plan below.





Existing Site

The existing site has moderate slopes that span the site from east to west. Runoff from the existing landscaping currently flows to the west of the site. The driveway will slope up to the proposed house location. A detention tank will be placed below the driveway portion to the east of the proposed house.

Proposed Storm Pump System

To maintain the existing stormwater flows, the runoff from the sloped driveway portion will be pumped to the detention tank where a control structure will limit the flows. The 100-year peak release rate of the trench drain is 0.009cfs/4.04 GPM, see WWHM modeling below.



A Grinder Pump Package system includes (2) 1/2 HP Grinder Pumps and a 36" x 72" fiberglass basin. The top of the fiberglass basin is at an elevation of 189.75'. The inlet of the pump in the fiberglass basin is at an invert elevation of 184.16' and will eventually discharge at an invert elevation of 190.33' in the Type 2 Catch Basin connected to the detention tank. Below is a summary of the proposed pump cycle and discharge velocity.

- Total Dynamic Head ≈ 6.17'
- Discharge Rate = 33 GPM
- Pump Cycle Minimum Storage Volume = 31.94 Gallons
- Time to Fill Minimum Storage Volume = 7.91 minutes
- Time to Discharge Minimum Storage Volume = 1.10 minutes
- Pump Cycle Time = 9.01 minutes
- Pump Cycles per Hour = 6.66



- Outlet Pipe: 1.25" Schedule 80 PVC
- Discharge Velocity = 8.62 ft/s

The proposed Grinder Pump Package system has a storage volume of 31.94 Gallons per pump cycle. Therefore, the pump will start after 7.91 minutes. Once the pump is turned on, the pump will run for 1.10 minutes before the "OFF" water level is peached. Subsequently, the cycle time is 9.01 minutes and there will be 6.66 cycles per an hour. See abbreviated pump specifications below.



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Product information presented here reflects conditions at time of publication. Consult factory regarding discrepancies or inconsistencies.

U.S. Patent No. 8,562,287



SECTION: 2.25.021 FM2882 0817 Supersedes 0716

TECHNICAL DATA SHEET SHARK GRINDER Model 803/805/807 Residential Grinder Pumps

PRODUCT SPECIFICATIONS

	Horse Power	0.5 - 1.0					
MOTOR	Voltage	115/230					
	Phase	1 Ph					
	Hertz	60 Hz					
	RPM	3400					
	Туре	Capacitor start / Capacitor run					
	Insulation	Class B					
	Amps	115 V (7-11) / 230 V (3-5)					
	Operation	Automatic & nonautomatic					
	Auto On/Off Points	13" (33 cm) / 5-3/4" (14.6 cm)					
	Discharge Size	1.25" NPT					
₫.	Cord Length	15' (5 m) standard					
N	Cord Type	UL listed 3-wire plug					
2	Max. Head	55' (16.8 m)					
	Max. Operating Temp.	130° F (54 °C)					
	Cooling	Oil					
	Motor Protection	Auto reset thermal overload (1 Ph)					
	Cover	Cast iron					
	Motor housing	Cast iron					
	Adapter	Cast iron					
10	Pump housing	Cast iron					
Ľ	Upper Bearing	Ball bearing					
M	Lower Bearing	Ball bearing					
E.	Mechanical Seals	Carbon and ceramic					
Ē	Impeller Type	Non-clogging vortex					
A	Impeller	Engineered plastic					
2	Hardware	Stainless steel					
	Motor Shaft	1215 cold rolled steel					
	Gasket	Neoprene					
	Cutter & Plate	440C Stainless Steel					



NON-AUTOMATIC



4 15/16" [125 mm]

NOTE: The sizing of effluent systems normally requires variable level float(s) controls and properly sized basins to achieve required pumping cycles or dosing timers with nonautomatic pumps.

NOTE: See model comparison chart for specific details.







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SK3



MODEL		8	03	8	05	807		
Feet Meters		Gal. Liters		Gal.	Liters	Gal. Liters		
5	1.5	35	132	40	151	45	170	
10	3.0	31	117	37	140	42	159	
20	6.1	22	83	29	110	35	132	
30	9.1	9	34	20	76	27	102	
40	12.2	-		8	30	18	68	
50	15.2		**		-	7	26	
Shut-off Head:		35 ft. (10.7 m)		45 ft. (1	3.7 m)	55 ft. (16.8 m)		



Model	MODEL COMPARISON										
	Seal	Mode	Volts	Ph	Amps	HP	Hz	Lbs.	Kg	Simplex	Duplex
M803	Single	Auto	115	1	7.0	0.5	60	65	29	1	3
N803	Single	Non	115	1	7.0	0.5	60	65	29	2	2&3
BN803	Single	Auto	115	1	7.0	0.5	60	65	29	4	3
D803	Single	Auto	230	1	3.0	0.5	60	65	29	1	3
E803	Single	Non	230	1	3.0	0.5	60	65	29	2	2&3
BE803	Single	Auto	230	1	3.0	0.5	60	65	29	4	3
M805	Single	Auto	115	1	9.0	0.75	60	65	29	1	3
N805	Single	Non	115	1	9.0	0.75	60	65	29	2	2&3
BN805	Single	Auto	115	1	9.0	0.75	60	65	29	4	3
D805	Single	Auto	230	1	4.0	0.75	60	65	29	1	3
E805	Single	Non	230	1	4.0	0.75	60	65	29	2	2&3
BE805	Single	Auto	230	1	4.0	0.75	60	65	29	4	3
M807	Single	Auto	115	1	11.0	1.0	60	65	29	1	3
N807	Single	Non	115	1	11.0	1.0	60	65	29	2	2&3
BN807	Single	Auto	115	1	11.0	1.0	60	65	29	4	3
D807	Single	Auto	230	1	5.0	1.0	60	65	29	1	3
E807	Single	Non	230	1	5.0	1.0	60	65	29	2	2&3
BE807	Single	Auto	230	1	5.0	1.0	60	65	29	4	3

SELECTION GUIDE

1. Integral float-operated mechanical switch, no external control required.

2. For automatic use single piggyback variable level float switch or double piggyback variable level float switch. Refer to FM0477.

3. See FM0486 for correct model of duplex control panel.

4. Single piggyback switch included.

CAUTION All installation of controls, protection devices and wiring should be done by a qualified licensed electrician. All electrical and safety codes should be followed including the most recent National Electrical Code (NEC) and the Occupational Safety and Health Act (OSHA).

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OUTDOOR BASINS AND ACCESSORIES

SIMPLEX OUTDOOR BASINS - FIBERGLASS ONLY - NO HOLES DRILLED

Size	Item No.	Description
24" X 48"	31-0866	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring
24" X 60"	31-0946	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring
24" X 72"	31-0594	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring
30" X 48"	31-1830	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring
30" X 60"	31-1831	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring
30" X 72"	31-1586	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring

All basins include 5 field installed adapta-flex seals - (1) 11/4", (1) 11/2", (2) 2" and (1) 4" grommets.

DUPLEX OUTDOOR BASINS - FIBERGLASS ONLY - NO HOLES DRILLED

Size WXH	Item No.	Description
36" X 48"	31-1450	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring
36" X 60"	31-1451	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring
36" X 72"	31-1452	Solid Fiberglass Basin with Solid Fiberglass Cover and Anti-Flotation Ring

All basins include 5 field installed adapta-flex seals - (1) 11/4", (1) 11/2", (2) 2" and (1) 4" grommets. ADDITIONAL BASIN SIZES WITH OPTIONS (i.e. Rail Studs) ARE AVAILABLE. CONSULT FACTORY.



E T UELLER _®				MAIL TO: SHIP TO: 3 (502) 778-	P.O. BOX 16347 • Lo 649 Cane Run Road • 2731 • 1 (800) 928-PU	uisville, KY 40256-0347 Louisville, KY 40211-1961 MP • FAX (502) 774-3624	visit our web site:
10-1464	Green	Metal	6" Female NPT	11.125" X 6.625"	28.3 sq. in.	42.5 sq. in.	
10-1463	Green	Metal	4" Female NPT	9.250" X 5.000"	12.6 sq. in.	35.8 sq. in.	
10-1462	Green	Metal	3" Female NPT	6.875" X 4.500"	7.1 sq. in.	19.6 sq. in.	
10-1461	Green	Metal	2" Female NPT	4.625" X 3.125"	3.1 sq. in.	6.9 sq. in.	
10-1753	Black	Plastic	2" Female NPT	4.625" X 3.125"	3.1 sq. in.	6.9 sq. in.	

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